City of St. Francis

PRIVATE DEVELOPMENT STANDARDS

April 27, 2023

Forward

In order to protect the public health, safety and welfare, it is necessary to establish standards for engineering in the City of St. Francis.

This manual outlines specific requirements, materials and standards that will be incorporated into the preparation of plans and specifications for sanitary sewer, storm sewer, watermain, trails, street construction and other improvements within the City of St. Francis. The following definitions shall be used for this manual:

Owner: Owner shall mean the person(s), company, corporation, etc. that enter into a "Developers Agreement" with the City of St. Francis for the purpose of construction of public improvements on lands under the ownership and control of said persons(s), company, corporation, etc.

Engineer: Engineer shall mean the Owner's Engineer.

City Engineer: City Engineer shall mean the Licensed Professional Engineer(s) under contract to the City to serve in that capacity.

Approved Plans: Shall mean all Plans and Specifications and information required to be shown thereon per the City of St. Francis Ordinances, along with these Standard Specifications.

Sanitary sewer facilities and water work shall be designed to conform to the "10 State Standards" and shall be constructed in accordance with City Engineers Association of Minnesota Standard Specifications except as modified by specific City of St. Francis requirements. Street and road surface improvements shall be designed to the standards of the Minnesota Department of Transportation design manuals and shall be constructed in accordance with the Minnesota Department of Transportation Standard Specifications except as modified by specific City of St. Francis requirements.

Development plans and public facilities construction plans shall conform to City of St. Francis Ordinances and Comprehensive Plans. Related to engineering, comprehensive plans include the sanitary sewer system with associated trunk facilities for area service, the water distribution system with watermain oversizing, the surface water runoff control plan and the city transportation plan with designated collector streets. The City of St. Francis has the authority to construct improvements as necessary conforming with City Comprehensive Planning with the costs of improvements allocated or assessed to properties for benefit.

Once the plat, plans and specifications and associated documents have been reviewed, approved and signed, the City will allow the Developers, as defined in the Development Agreement, to proceed with the construction.

These standards are established as policy and as such may be subject to change by action of the City Council. The City of St. Francis Private Development Standards manual was approved by the City Council on ______.

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GLOSSARY OF TERMS

AASHTO	American Association of State Highway and Transportation Office
ANSI	American National Standards Institute
ASTM	American Society of Testing and Materials
AWWA	American Water Works Association
CEAM	City Engineer's Association of Minnesota
CMP	Corrugated Metal Pipe
HDPE	High Density Polyethylene
MnDOT	Minnesota Department of Transportation
MnPCA	Minnesota Pollution Control Agency
PID	Property Identification Number
PVC	Polyvinyl chloride
RCP	Reinforced Concrete Pipe
SDR	Strength to Diameter Ratio

ENGINEERING MANUAL

I. Engineering Requirements

As set forth in various sections of the City ordinances, Developers of property within the City of St. Francis are required to submit certain plans and specifications for review and approval by the City. These include such items as grading plans, drainage plans, topographic surveys, plats, street and utility plans and specifications. These plans and specifications shall be prepared by competent professionals.

The professional services required of the Developer may include one or more of the following professionals: architect, land surveyor, planner, soils and civil engineer and testing service. The engineering services include not only preparation of plans and specifications, but field staking in order to assure the City that the completed project is in conformance with the approved plans and specifications. The City will provide construction observation of the installation of the facilities at the Developer's expense.

The following procedures shall be followed:

- 1. The Developer shall submit plans, specifications and copies of all design calculations to the City for review and approval. These plans are to be prepared by a licensed professional civil engineer and shall be in accordance with City standards as outlined herein. The City comprehensive sanitary sewer, water, storm drainage and thoroughfare plans shall be adhered to in design considerations. All sanitary sewer and watermain testing shall be completed and copies of service ties submitted to the City prior to issuance of any service connection permits.
- 2. The Developer shall submit erosion and sediment control plans to the City for review and approval. No work is to begin until all erosion and sediment control methods are in place and approved by the City.
- 3. The Developer shall furnish a separate Development Plan showing housepad with elevations on front & back of pad and garage floor slab, lot corners, drainage arrows, and street grade in front of driveway.
- 4. The Developer will be responsible for not only plans and specifications preparation, but also for providing construction staking. Resident construction observation of said improvements to assure compliance with the approved plans shall be completed by the City.
- 5. Copies of all bids, change orders, etc. relating to the improvements shall be forwarded to the City Engineer.

- 6. The Developer shall furnish to the City the list of selected contractors and subcontractors being considered for retention by the Developer for any of the public improvements work in the development. The City has the right to reject any contractor or subcontractor deemed unacceptable to the City.
- 7. Any changes to the approved plans and specifications shall be approved by the City Engineer in writing before work is started. If the change affects the project letter of credit by increasing the cost, the letter of credit shall be increased before the work can begin.
- 8. The Developer will hold a preconstruction meeting at the City Hall prior to start of any work on the development. The City staff and City Engineer along with the contractor and subcontractors, Developer's engineer, utility companies and other interested parties must be invited to the meeting. The Developer will be responsible for drafting pre-construction meeting minutes. The said minutes shall be submitted to the City Engineer for review, and then distributed by the Developer to all parties whom were in attendance at the meeting.
- 9. The Developer shall retain an independent testing service to perform the required tests of materials. Copies of tests will be directed to the City Engineer or his designated representative. The cost of this service will be the responsibility of the Developer.

The City shall be notified 24 hours in advance of all scheduled tests so its representatives can be present at the time tests are made. The required tests include sanitary sewer, watermain, storm sewer, street subgrade, base course, wear course, and curb and gutter.

- 10. Upon completion of all the work required, the City Engineer or his/her designated representative, a representative of the contractor and a representative of the Developer will make the required final inspections of all work. This includes a final inspection of all site grading and approval by the City Engineer before any building permits will be issued. Before the improvements are considered for acceptance by the City, the City Engineer shall be satisfied that all work is satisfactorily completed in accordance with the approved plans and specifications, and the Developer's engineer shall submit a written statement attesting to same. Acceptance of the completed work shall be made by motion of the City Council upon the recommendation of the City Engineer.
- 11. An as-built survey signed by a Professional Land Surveyor for each individual lot will be required to verify lot corner elevations, swales, emergency overflow elevations, and house low floor and lowest opening elevations. Said survey shall be submitted to the City Building

Department for review and approved by the City Engineer prior to the Certificate of Occupancy being issued.

12. Warranty Period – If within the time prescribed by law, by the contract documents and/or the Developer's Agreement any of the work is found to be unacceptable, the Developer shall correct it promptly unless the City Council has previously accepted the work. The Developer shall give prompt notice after discovery of any unacceptable conditions to the contractor responsible for the project work.

Unless otherwise noted in the contract documents, the following requirements shall apply:

- a. The contractor shall guarantee all work relating to street construction including concrete curb and gutter, utilities, appurtenances, material and equipment furnished by him/her for a period of <u>two</u> years from the date of written acceptance by the City Council of the work or project.
- b. The Developer shall provide letter of credit(s) as defined in the Developers Agreement. The amount of the letter of credit(s) will be determined by the City Engineer and the City Attorney.
- 13. After all public improvements have been completed, properly inspected as specified above, and after an acceptable maintenance guarantee has been provided the public improvements will be scheduled for acceptance by the City Council subject to the following:
 - a. The Developer or the Developer's engineer must submit written certification to the City Engineer stating that all public improvements have been completed in accordance with the approved plans and specifications.
 - b. The Developer's engineer shall provide the City with a complete set of "as-builts" for the City records as outlined in this manual. These as-builts shall be submitted prior to release of building permits.

II. <u>Erosion Control Policy</u>

- 1. <u>Required Erosion Control Plan.</u> Prior to commencing any earth disturbing activity in a subdivision, the Developer shall prepare and submit to the City Engineer a Stormwater Pollution Prevention Plan (SWPPP).
- 2. The SWPPP shall conform to the MnPCA's "Application for General Storm-water Permit for Construction Activity (MN R100001) and as specified herein:

- a. The plan shall be suited to the topography and soils so as to create the least erosion potential.
- b. The land shall be developed in increments of workable size on which adequate controls of erosion and siltation can be provided and maintained during the construction period. Grading operations and other land disturbing operations shall be staged so that the area being developed is not exposed for long periods of time without stabilization.
- c. Temporary vegetation and/or mulching shall be used to protect the areas exposed during the development. No area shall be left denuded for a period longer than five (5) days after initial site grading and other land disturbing operations on slopes of 3:1 and greater. These areas shall be mulched and stabilized with an erosion control netting or hydraulic erosion control matrix acceptable to the City Engineer.
- d. Permanent vegetation and erosion control measures shall be installed within time period as prescribed in the approved SWPPP. If grading is not completed until after the planting season has expired, temporary erosion control measures, including dormant seeding and mulching, shall be implemented.
- e. Sediment basins (debris basins, desilting basins, or silt traps) shall be installed and maintained to remove sediment from runoff waters from the land undergoing development. Storm sewer inlets shall be provided with debris guards and microsilt basins to trap sediment and avoid possible damage from blockage. The silt shall be removed when necessary. If sediment/siltation measures taken are not adequate and result in downstream sediment, the Developer shall be responsible for cleaning out or dredging downstream storm sewers, ponds, and/or natural features as necessary.
- f. Before grading is commenced, all control measures as shown on the approved plan shall be installed.
- g. Immediately after curb and gutter has been placed, cured, and backfilled, approved erosion control measures shall be installed directly behind the curb.
- h. Erosion control practices shall comply with the Minnesota Pollution Control Agency Best Management Practices.

- i. The Developer shall be responsible for cleaning and maintenance of the storm sewer system (including ponds, pipes, catch basins, culverts, and swales) within the subdivision and the adjacent offsite storm sewer system that receives storm water from the subdivision. The Developer shall follow all instructions it receives from the City concerning the cleaning and maintenance of the storm sewer system. The Developer's obligations under this paragraph shall end as defined in the Developers Agreement.
- j. The Developer shall be responsible for cleaning all streets in the subdivision and adjacent to the subdivision from sediment and debris from the subdivision for a period of two (2) year beginning when the streets have been completed and accepted by the City.

3. <u>Financial Guarantee</u>

- a. A portion of the Developer's letter of credit required by the Developer's agreement shall include a guarantee of compliance with erosion control measures, and shall be furnished upon approval of the Developer's Agreement before work is commenced. The financial guarantee shall remain in place until all the Developer's obligations under the erosion control plan have been satisfied.
- b. If the City draws upon the financial guarantee, the Developer shall within ten (10) days of the draw, deposit with the City additional security in the same amount that the City has drawn. No further inspections will be conducted, no new building permits will be issued, and all work must stop within the development until the cash deposit for erosion control is restored to the predraw balance.
- 4. <u>Street Sweeping.</u> The Developer shall provide street sweeping within the plat before the final acceptance is approved. If the construction operation within or out of the plat causes debris on the existing streets, the City Engineer may require street sweeping done by the Developer.

5. <u>Enforcement.</u>

- a. The City may issue a stop work order halting all development work and building construction for noncompliance with the erosion control plan.
- b. The City may draw down the posted financial guarantee and perform any work necessary to achieve compliance with the erosion control plan. The City will endeavor to give the Developer advance notice of such action.

III. <u>City Standard Plans</u>

In order for the City to have standardized construction and as-built plans, the guidelines listed below shall be followed:

General Requirements:

- 1. The Developers must consider the requirements for plans found in the subdivision ordinance and street construction standards attached herein.
- 2. Incorporated in the set of plans shall be a sheet indicating the entire project, with corresponding sheet numbers on each separate sheet and index.
- 3. All sheets shall be 22" x 34", reduceable to 11" x 17" at correct scaling.
- Scale Horizontal Scale 1" = 50'
 Vertical Scale 1" = 5'
 (unless otherwise approved by the City engineer)
- 5. General Details
 - a. North arrow
 - b. Scale with bargraph
 - c. Date of preparation
 - d. Proposed name of the subdivision in which the roadway and utilities are to be constructed.
 - e. Proposed name of all streets
 - f. Name of the plan preparer, Engineer, Surveyor and Owner
 - g. Seal or signature of the preparer and Licensed Engineer
 - h. Street, sanitary sewer, watermain and storm sewer plan and profile shall be drawn at a scale of 1" = 50' horizontal and 1" = 5' vertical.
 - i. Street cross-sections shall be drawn at a scale of 1" = 10' horizontal and 1" = 5' vertical.

- j. Location map which shows all existing streets within 2500 feet of the proposed plat.
- 6. All utilities shall be shown in the following approximate locations:
 - a. Sanitary Sewer on centerline of street right-of-way
 - b. Watermain ten feet north and east of centerline
 - c. Storm Sewer ten feet south and west of centerline
- 7. All detail drawings shall be on a separate sheet and referenced to the proper sheet.
- 8. The profile shall be directly below the plan with the stationing aligned as closely as practical. Stationing shall be shown on the plan view as well as the profile.
- 9. All parcels shall be properly labeled with lot and block numbers and plat name, or P.I.D. in unplatted areas. Developed parcels shall have their address shown on the plan. Bearings and distances for all existing roadway centerlines and right-of-ways described above shall be shown.
- 10. All match line breaks shall be clean with reference points clearly marked. All plans which are broken by a matchline shall be on the same or consecutive sheets.
- 11. Existing utilities shall be shown in both plan and profile, stationed and labeled as existing.
- 12. Approximate locations of gas, electric, telephone and cable lines shall be shown.
- 13. Right of way and pavement or curb and gutter alignment data shall be shown. Right of way shall be rounded at intersections to allow for utility installation.
- 14. Bench marks shall be placed on all sheets.

Specific Requirements:

- 1. Stationing of sanitary sewer wyes shall be indicated "S" in front of the stationing.
- 2. All sanitary sewer services shall be drawn on the plan to the constructed length. If other than open cut trench methods are employed, the method needs to be indicated on the plans.

- 3. If the sanitary sewer wye only is constructed, it shall be noted as "Wye Only" after stationing.
- 4. The invert elevation of all sanitary sewer services shall be shown on the plans. If risers are installed, the height of each shall be indicated on the plans and also drawn on the profile, along with the height of each riser.
- 5. All manholes shall be numbered on both plan and profile.
- 6. All hydrants, gate valves and tees shall be stationed on the bottom of the profile.
- 7. All water corporation stops shall be indicated by a "W" in front of its stationing.
- 8. All water services shall be drawn to constructed length. If other than open cut service installation methods are employed, the method needs to be indicated on the plans.
- 9. The size and type of materials of all sanitary sewer and water services shall be noted on the plans.
- 10. On combination sewer and water projects, services may be placed in the same trench with sanitary sewer services three feet downstream from water services. Locations will be noted on the plans with an "S & W" in front of the stationing.
- 11. All sewer and watermain shall be shown in the profile with the appropriate information such as size, material, grades, invert elevations, etc.

As-Built Requirements:

- 1. All as-built plans shall be submitted electronically in .pdf and .dwg format.
- 2. As-built plans on all ponding areas are required. Plans shall indicate asbuilt spot elevations overlaid on the proposed contours, normal water elevation, high water elevation, and the acre feet of storage for each ponding area along with the final storm sewer plans.
- 3. All "as-built" plans shall be certified by the design engineer and land surveyor responsible for the field work.
- 4. All water valves shall be located with at least two permanent field ties, using the following priority:
 - a. Fire hydrants

- b. Manholes
- c. Catch basins, if curb and gutter is in
- d. Buildings or other permanent structures
- e. Power poles, trees, other semipermanent items
- f. Stationing from hydrants, manholes, catch basins, if over 100'
- g. Back of curb only when used with station in (f.) above
- 5. All services shall be tied with at least two ties, using the following priority:
 - a. The served structure with address noted
 - b. Neighboring structures with address noted
 - c. Fire hydrants
 - d. Manholes, catch basins, if curb and gutter is in
 - e. Other permanent structures (bridges, telephone boxes, electrical boxes, etc.)
 - f. Power poles, trees, and other semipermanent items
 - g. Stationing from hydrant, manhole, catch basins these may be used with back of curb distance only as last possible means.
- 6. Show contractor's name on the as-builts.
- 7. Show where fabric has been placed or correction to pavement section has been made in the streets on the plan portion of the as-builts.
- 8. Benchmarks shall be referenced on each sheet.
- 9. All hydrants are to be at required height after lawns, boulevards, etc. are finished (sod, seed, etc.) <u>This will be the Developer's responsibility.</u>
- 10. The Developer shall provide the City with linework and feature locations as necessary for updating the City's G.I.S. mapping. The Developer shall pay the costs for updating the G.I.S. mapping to include the new infrastructure.

IV. <u>City Standard Materials</u>

In order to standardize certain construction materials and assure quality construction, we have adopted the following:

- 1. Sanitary Sewer pipe and service line materials:
 - a. Plastic pipe shall be smooth wall polyvinyl chloride (PVC) and shall conform with ASTM D 3034 for the size and strength requirements shown on the plans. Minimum pipe strength shall be SDR 35 for depths 20 feet and less. For sewer depths greater than 20 feet, the pipe shall be SDR 26. In general, sanitary sewer shall not exceed

26 feet in depth unless approved by the City. All joints shall be elastomeric gasketed.

- b. Ductile iron sewer pipe shall be Class 50 and shall meet ANSI specifications A-21.51.
- c. All connections between existing and new sanitary sewer or service pipe shall be made with factory manufactured flexible couplings, Fernco or equivalent, specially designed and sized for sanitary sewer connections.
- d. All PVC sanitary sewer service pipe and fittings shall be SDR 26 minimum pipe strength. All service pipe connections shall be solvent welded. Gasketed connections will not be allowed.
- e. Maximum distance between sanitary sewer manholes shall be 400 feet. All sanitary sewer manholes shall be located on the street centerline unless otherwise approved by the City.
- f. Green tracer wire is required along all sanitary sewer main and service pipe in accordance with the specification as contained herein.
- 2. Storm Sewer and Drainage Pipe
 - a. All storm sewer pipe within any street right-of-way shall be reinforced concrete pipe of the class as shown on the plans. Pipe shall meet Mn/DOT 3236 Specification. Pipe joint sealer materials shall be preformed rubber, Type A, in accordance with Mn/DOT Specification Section, 3726.
 - b. High density polyethylene (HDPE) is not allowed.
 - c. Riprap shall be required for all sizes to prevent erosion. Fabric blanket conforming with MnDOT 3733 Type IV shall be required under the rip-rap. Erosion control blanket is required at all inlets per Mn/DOT Standard Plat No. 9102D.
 - d. All flared end sections for pipe culverts 18" and larger shall be fitted with trash guards and all flared end sections on pipe storm sewer systems shall be fitted with trash guards.
- 3. Metal Sewer Castings
 - a. Castings for sanitary sewer manhole shall be Neenah R1733 or approved equal with a concealed pickhole and a neoprene gasket

and groove for watertight application. The words "Sanitary Sewer" shall be imprinted on the cover. Waterproof castings where required shall be Neenah R 1755 or approved equal.

- b. Castings for storm manholes and catchbasin shall be in accordance with the standard plates and schedule of structures. Unless otherwise specified, castings shall be equivalent to Neenah R-1733 for manholes and R-3067 with V or VB grates for catch basins.
- c. Off-street catchbasin manhole and catchbasin inlets shall be constructed per MnDOT Standard Plate 4143E Stool Grate & Concrete Frame and shall utilize a R-4342 or equivalent grate.
- 4. Manhole and Catchbasin Structure
 - a. Manhole and catchbasin structure shall be in accordance with applicable MnDOT standard plates or City standard plates and Mn/DOT Standard Specification Section 2506. All manholes and covers shall be reinforced for traffic loadings.
 - b. Manholes identified on the plans as box structures shall be constructed from precast reinforced concrete box sections conforming to ASTM C-789 placed on end. Wall thickness and reinforcement shall be in accordance with ASTM C-789 Table 1 for box section under earth dead load and HS-20 live load conditions. Base and cover slabs shall have thickness and reinforcement to meet MnDOT HS-20 traffic loadings.
 - c. All manhole and catchbasin structures with builds greater than 5.0 feet from casting to invert shall have steps. Maximum distance from top of casting to first step is 2 feet.
 - d. Sanitary sewer adjusting rings shall be Ladtech HDPE or approved equal. Storm sewer adjusting rings shall be concrete with approved Chimney Seal or Infi-Shield External Seal or Flex-Seal Utility Seal.
- 5. Chimney Seal or Infi- Shield External Seal or Flex-Seal Utility Seal

Any one of the three following ring seals is allowed. (Contractor choice)

- a. CHIMNEY SEAL
 - 1. Chimney seals shall consist of a flexible internal rubber sleeve, interlocking extensions and stainless steel expansion bands. (See standard plate No. 308)

2. The seal shall remain flexible throughout a 25 year design life, allowing repeated vertical movement of the frame of not less than 2 inches and/or repeated horizontal movement of not less than $\frac{1}{2}$ inch. The sleeve portion of the seal shall be either double or triple pleated with a minimum unexpanded vertical height of either 8 inches or 10 inches respectively. The sleeve and extension shall have a minimum thickness of 3/16 inches and shall be made from a high quality rubber compound conforming to the applicable requirements of ASTM C-923, with a minimum 1500 psi tensile strength, a maximum 18% compression set and a hardness (durometer) The bands shall be integrally formed from 16 of 48+5. gauge stainless steel conforming to ASTM A-240, Type 304, with no welded attachments, shall have a minimum adjustment range of 2 diameter inches and a positive locking mechanism. Any screws, bolts or nuts used for this mechanism shall be stainless steel conforming to ASTM F-593 and 594, Type 304.

b. FLEX-SEAL UTILITY SEALANT

1. Manhole seal shall be designed to prevent leakage of water into the manhole through the frame joint area and the area above the manhole cone including all extensions to the chimney area. Extensions shall include but is not limited to lifting rings, brick and/or block material that may have been used to achieve grade. The seal shall remain flexible allowing for the repeated vertical or horizontal movements of the frame due to frost lift, ground movement or the thermal movement of pavements. The final liner material shall be made no less than 200 mils. of corrosion resistant aromatic flexible urethane resin coating to be applied to the inside wall of the entire chimney area as described above. The product shall have a minimum elongation of 800% and a hardness (Durometer) of 75. Final liner shall have a minimum tensile and adhesion strengths of 1150 psi and 175 lb. l/in. respectively. The manhole sealing system shall conform to the physical requirements of ASTM D-412, with a minimum of 200 mils. thickness for durability and resistance elongation and tearing. The lining product shall have an aromatic urethane primer resin on the complete surface. The sealing system shall line the interior of the adjustment area from the cone/top of the manhole and onto the inside of the casting. (See standard plate No. 309)

- 2. All lose and protruding mortar and brick that would interfere with the seal's performance shall be removed. Any lips for gravel pan supports shall be cut off flush with casting. All excessive voids shall be sealed. Patching cement, shall conform to the manufactures requirements. Any patching cement work will require the contractor to contact the sealant manufacture to determine in writing the proper time required for the cement to completely cure prior to installing this item. Preparation of surface shall include water blasting machine that delivers the water with a sandblaster attachment in a steady stream at a minimum of 3500 psi. Surface preparation shall also include wire brushing of surface to ensure a clean surface as required by manufacture. Active leaks (infiltration) should be corrected by a method approved by the City Engineer prior to installing an Internal Manhole Seal. After water sandblasting, pressure wash the entire area to remove any loose sand that may have been deposited. The substrate surface must be free of sand, loose debris, latencies, dust, oil, grease or chemical contamination. A blower may be required to completely dry the substrate surface or as recommended by manufacture. Ensure casting and structure surfaces are clean and dry where the primer is intended to adhere. Flex-Seal Utility Sealant or approved equal may require the proper mixing of agents, as recommended by the manufacture's instructions. After allowing for proper drying of primer to occur, sealant may be applied by brush as evenly as possible over the entire chimney area, that includes the frame joint area and the area above the manhole cone including all extensions to the chimney area. The contractor is to furnish the City Engineer two (2) mirrors with extension handles that can be used to inspect sealant application to areas underneath frame without entry of manhole. These items will become the property of the owner upon completion and at no additional cost of this item. Cost for these items shall be included in the bid items for internal manhole sealing work.
- 3. The manufacture must in writing certify that each of the contractor's representatives are approved to install item. The proof or certification of training shall be included in the bid items for internal manhole seals.
- c. INFI-SHIELD
 - 1. The casting shall be sealed to the structure with an external sealing system. The seal shall be continuous bands, made

of high quality EPDM (Ethylene Propylene Diene Monomer) rubber with a minimum thickness of 60 mils. Each unit shall have a 2" wide mastic strip on the top and bottom of the band. The mastic shall be non-hardening butyl rubber sealant, with a minimum thickness of 3/16", and shall seal to the cone/top of the manhole section and over the flange of the casting. (See standard plate No. 310)

- 2. The external sealing system shall be installed according to the manufacturers recommendations. The external seal shall extend onto the casting and the cone section a minimum of 2".
- 6. Watermain and Appurtenances

Materials shall conform to the Standard Specifications and to the following:

a. Watermain shall be polyvinyl chloride (PVC) pressure pipe. PVC watermain shall conform to AWWA C 900 for pipe sizes 4" to 12" and AWWA C 905 for pipe sizes 14" to 24". All pipe shall have a minimum dimension ratio (DR) of 18 corresponding to a working pressure of 150 psi for PVC type 1120 pipe.

The bell of the joint shall consist of an integral wall section with a factory installed, solid cross section elastometric ring, which meets the requirements of ASTM F 477. The bell section shall be designed to be at least as hydrostatically strong as the pipe wall and meet the requirements of AWWA C 900 for sizes 4" to 12" and AWWA C 905 for sizes 14" to 24". The pipe shall be manufactured to ductile iron outside dimensions in accordance with AWWA C 900.

Restraints for C 900 and C 905 PVC pipe shall be Ebba Iron Series 2001PV, Uniflange Series 1300C, and 1390C or approved equal. Fittings for the PVC watermain shall be ductile iron.

Blue tracer wire shall be laid with all PVC watermain in accordance with the specifications included herein.

Granular bedding material shall be furnished and installed with the PVC watermain and associated services, as necessary.

b. Bends shall be 45 degrees or less. Any deflection greater than 45 degrees shall be made with multiple bend sections.

c. Valves 12" and larger shall be butterfly valves, AWWA C504, Mueller Line Seal or approved equal. Valves smaller than 12" in size shall be resilient wedge valves, Mueller A 2360 Series, American-Darling Series 2500 or approved equal conforming to AWWA C509 standards. All valves shall be installed on-line with accompanying valve boxes. All valves shall close in a clockwise direction. All valves shall be epoxy coated as per AWWA C550.

Valve boxes shall be three piece adjustable screw type boxes, nominal 60" to 90" extension, with a 5 ¼" shaft diameter. Valve boxes shall be provided with extension suitable for the design location and a minimum 6-inch available adjustment after final setting. The word "Water" shall be imprinted on each lid.

All valves shall be fitted with extension rods to within one foot of the finished ground surface.

- d. All fire hydrants shall be Clow Medalliaon ® F2545 with 16" break off section. The hydrant lead shall contain a 6" gate valve. Color and threads shall match City of St. Francis Standards. A Hydrafinder® red and white stripe shall be included. 8'6" bury depth typical. All hydrants shall be purchased from licensed distributors of Clow hydrants. All hydrant bolts shall be stainless steel.
- e. All water service will be minimum 1-inch. All copper service pipe shall be Type K or Polyethylene Grade PE-3408 or PE-4710, or High Density Polyethylene SDR 11 and shall be rated for 200 PSI working pressure.
- f. The component parts of a tap service installation shall include a corporation stop coupling complete with watermain tap and saddle; a curb stop coupling complete with curb box; and copper service piping extending from the corporation stop to the curb stop coupling. Corporation stops for 1" through 2" services shall be Mueller 300 Ball Type or Ford FB-600-4-NL Series Ballcorp. All services shall be wet tapped. Service saddles shall be all Type 304 stainless steel Smith-Blair 372 TaperSeal service saddles, or approved equal. Curb stops for 1" through 2" service shall be Ford Ball Valve B22-444M-NL Series or Mueller Oriseal and shall be a Minneapolis pattern valve with thread top. Curb box shall be Minneapolis base, sized to fit the curb stop. Boxes shall have a one and one-quarter (1 1/4) inch upper section and shall be furnished with a stationary rod 66" in length. All curb boxes shall utilize a BoaBox with tracer wire and ground wire securely connected to the BoaBox terminals. All curb boxes located beneath

driveways, shall have Ford Series A lid covers placed over the riser. (See Standard Plate 207)

- g. Water services 2-inches and larger shall be constructed with pipe, fittings, valves and boxes as specified for PVC pipe installation.
- h. Water meter shall be obtained from the City with payment of established fee, and installed in accordance with directions.
- 7. Street Material

All materials shall be in conformance with Minnesota Department of Transportation Standard Specifications for Construction, latest edition and all subsequent revisions (MnDOT) or as modified herein in Appendix C.

V. <u>Testing Requirements</u>

Materials shall be sampled and tested in accordance to the MnDOT schedule of material control, except for as modified below. Utility systems shall be tested in accordance with the standard specifications for watermain, service lines, sanitary sewer and storm sewer as published by the City Engineer Association of Minnesota. The City Engineer shall be notified 24 hours in advance of the specific test.

- 1. <u>Pipe Trench Compaction:</u>
 - a. <u>Standard Proctor Density (ASTM D-698-78)</u>: Proctor samples will be obtained within the utility trenches for each type of soil encountered in construction.
 - b. <u>Density Test Nuclear (ASTM D-2922): 1 test per lift of backfill, 1 test</u> <u>every 500 feet of pipe installed, minimum 1 test daily when backfilling.</u>
 - c. <u>Sand-Cone Method (ASTM D-1556)</u>: The City Engineer may at his or her discretion, order density tests by the sand cone method.

- 2. <u>Embankment Compaction:</u>
 - a. <u>Standard Proctor Density (ASTM D-698-78)</u>: 1 test per source of material.
 - b. <u>Density Test Nuclear (ASTM D-2922)</u>: 1 test per lift of embankment, 1 test every 500 feet of roadway fill, 1 test daily when constructing embankment.
 - c. <u>Density Test Sand-Cone Method (ASTM D-1556)</u>: The City Engineer may, at his or her discretion, order density tests by the sand cone method.
- 3. <u>Select Granular Borrow</u>
 - a. <u>Standard Proctor Density (ASTM D-698-78)</u>: 1 test per source of material.
 - b. <u>Gradation Test:</u> 1 test per source of material.
 - c. <u>Density Test Nuclear (ASTM D-2922)</u>: 1 test per lift of embankment, 1 test every 500 feet of roadway fill, 1 test daily when constructing embankment.
 - d. <u>Density Test Sand-Cone method (ASTM D-1556)</u>: The City Engineer may, at his or her discretion, order density tests by the sand cone method.
- 4. <u>Concrete Tests</u>
 - a. <u>General:</u> When molding cylinders for strength tests, three cylinders are to be made according to ASTM C-31.
 - b. <u>Compressive Strength (ASTM C-39)</u>: 1 set of 3 for every 1000 l.f. of curb and gutter constructed or 1 set of 3 for every 100 cubic yards of concrete placed or a minimum 1 set of 3 daily when pouring concrete.
 - c. <u>Percent Air Test (ASTM C-231)</u>: 1 test for every 1000 l.f. of curb and gutter constructed or 1 test for every 100 cubic yards of concrete placed or a minimum 1 test daily when pouring concrete.
 - d. <u>Slump Test (ASTM C-143)</u>: 1 test for every 1000 l.f. of curb and gutter constructed or 1 test for every 100 cubic yards of concrete placed or a minimum 1 test daily when pouring concrete.

5. <u>Televising</u>

The Developer shall televise all sewer pipe, pipe joints, and service connections. One copy of the televising report and tape shall be submitted to the City for review.

VI. <u>Construction Requirements</u>

- 1. Sanitary Sewer, Watermain, and Storm Sewer
 - a. <u>Applicable Specifications</u>

Work shall conform to the Standard Utility Specifications as published by the City Engineers Association of Minnesota, latest edition.

2. Storm Sewer

Pipe sewers shall be installed in accordance with CEAM 2621 and MnDOT 2501 and 2503, except as modified by these specifications.

3. Casting Adjustments

All utility castings shall be adjusted as follows:

a. Sewer Manhole:

All sanitary and storm sewer manhole castings shall be in place during the laying of the wear course. The castings shall be adjusted before the mat is laid and shall be not less than one-eight inch (1/8") nor more than three-eighths inch (3/8") below finished grade.

b. Storm Sewer:

Storm sewer inlet castings shall be adjusted to be 0.1 feet below finished gutter line.

c. Water Valve Boxes:

All water valve boxes shall be adjusted prior to wear course paving and shall be not less than one-eight (1/8") nor more than threeeights inch (3/8") below finished grade. Only screw-type adjustments are allowed. d. Grouting Adjusting Rings:

Whenever adjustment rings are provided, the contractor shall grout rings, place the castings and remove all excess grout on the inside and outside of the manhole by wiping smooth with a gloved hand or similar instrument.

4. Streets

The street shall be constructed in accordance with typical sections shown on City Standard Plates and specifications as approved by the City Engineer. The final wear course shall not be constructed until at least one construction season after the base construction is completed.

VII. Storm Water Treatment Basins

1. Storm water conveyance, storage and treatment basins shall be designed in accordance with the City of St. Francis's policy on stormwater drainage in Appendix B. Typical basin construction and outlet structures are shown on the City Standard Plates in Appendix A.

VIII. <u>Miscellaneous</u>

- 1. Proper notification of improvements shall be given by the Developer or his/her engineer to the responsible governmental agencies, watershed districts, etc. affected by said construction. All necessary permits shall be obtained prior to commencing any work. All special requirements of the responsible agencies shall be complied with.
- 2. The Developer's contractor shall furnish, erect and maintain signs and barricades as provided in MnDOT 1710 "Barricades and Signs" under the General Conditions to protect the public. The City Engineer shall be notified 48 hours prior to the proposed partial blockage or closure of any street or public right-of-way. No street or public right-of-way shall be closed without the proper approval of the City Engineer.
- 3. It is the responsibility of the Developer's contractor to protect and leave undisturbed those markers or monuments set for the subdivision of land.
- 4. The Developer and/or his/her contractor shall immediately repair or replace at his/her own expense any defective workmanship or material of which he/she is notified during the construction period, or within the warranty period, regardless of the approval and acceptance of the work.

- 5. A plan for the routing of construction traffic shall be submitted to the City Engineer for his/her approval. City streets that are utilized for access or egress to the construction site shall be kept free of dirt and other debris resulting from said construction. Adequate control of dust shall be maintained by the Developer or contractor.
- 6. The City will require the contractor or Developer to submit a list of materials and respective suppliers as well as all tests of materials.
- 7. If any material or labor supplied by the contractor or Developer is rejected by the City Engineer or his/her designated representative as defective or unsuitable, then such rejected material shall be promptly removed, disposed of off the job site, and replaced with approved material.
- 8. All street right-of-ways shall be cleared and grubbed to full width except as specifically directed.
- 9. The standard ten (10) foot utility and drainage easement adjacent to the street right-of-way shall be cleared and grubbed for the placement of utilities except as specifically directed.
- 10. Work shall not commence before 7:00 a.m. nor extend beyond 7:00 p.m. Monday through Friday. On Saturdays, the hours will be from 8:00 a.m. to 6:00 p.m. No work is to be done on Sundays. Hours and days of work may be modified based on need.

APPENDIX A

APPENDIX A

SERIES 1 PAVEMENT

- 100 Local Residential Urban Street Section 9 Ton
- 101 Through Local Residential Urban Street Section 9 Ton
- 102 MSA Collector Urban Street Section 10 Ton
- 103 Rural Residential Street 9 Ton
- 104 Residential Cul-De-Sac Urban Section
- 105 Residential Cul-De-Sac Rural Section
- 106 Temporary Cul-De-Sac Urban Section
- 107 Bituminous Patch Section
- 108 Minimum Public Street Standards (Table)
- 109 Rural Driveway Standards
- 110 Local Residential Private Street Section Ending in Cul-De-Sac
- 111 Local Residential Private Street Section Thru

SERIES 2 WATER SYSTEM AND APPURTENANCES

- 200 Water Service Detail (2" and Less)
- 201 Water Service Detail (Greater than 2")
- 202 Water Service Detail Boulevard
- 203 Thrust Block Detail
- 204 Watermain Concrete Blocking Quantities
- 205 Clow Medallion Hydrant
- 206 Hydrant & Valve Installation
- 207 Typical Resilient Wedge Valve & Box Installation 10" & under Watermain
- 208 Typical Butterfly Valve & Box Installation 12" & Over Watermain
- 209 Curb Stop Cover for Driveway Installation
- 210 Hydrant Tracer Wire Detail
- 213 PVC C900 Watermain Trench
- 214 Watermain Offset
- 215 Gate Valve Alignment Device
- 216 Gate Valve Extension Stem
- 217 Tracer Wire Sample Water Plan

SERIES 3 SANITARY SEWER AND APPURTENANCES

- 300 Sanitary Sewer Standard Manhole
- 301 Water Tight Seals
- 302 Shallow Sanitary Main Service Connection
- 303 Deep Sanitary Main Service Connection
- 304 Insulation for Water & Sanitary Sewer Pipe & Services
- 305 Standard Drop Manhole
- 306 Pipe Jacking Detail
- 307 Sanitary Sewer Manhole Frame and Casting
- 308 Internal Chimney Seal
- 309 Manhole Adjusting Rings
- 310 External Chimney Seal
- 312 Forcemain Air/Vacuum Valve

- 313 Forcemain Cleanout
- 314 Manhole Joint Seal
- 315 Tracer Wire Plan (Sewer)
- 316 Tracer Wire Service Detail
- 317 Tracer Wire Sewer Manhole Detail

SERIES 4 STORM SEWER APPURTENANCES

- 400 Slab-Top Manhole Storm Sewer
- 401 Storm Sewer Manhole Deeper than 15'
- 402 Skimmer Structure
- 403 Skimmer Structures with Weir
- 404 Typical Treatment Basin
- 405 48 Inch Diameter Shallow Catch Basin
- 406 Standard Storm Manhole Catch Basin
- 407 Plate Style Grate for 48" Dia Outlet Structure
- 408 Standard Storm Manhole Yard Inlet
- 409 27" Precast Catch Basin Yard Inlet
- 410 2' x 3' Catch Basin
- 411 Transverse Permeable Aggregate Base (PAB) Drain
- 412 Longitudinal Permeable Aggregate Base (PAB) Drain
- 414 Storm Manhole Adjustment Rings
- MnDOT4143E Stool Grate & Concrete Frame
- MnDOT 4180J Manhole or Catch Basin Step

SERIES 5 EROSION CONTROL AND LAND APPURTENANCES

- 500 Articulated Concrete Block at RCP Outlets
- 501 Articulated Concrete Block Mat Layout
- 502 Rip Rap at RCP Outlet
- 503 Silt Fence Installation
- 504 Erosion Control Blanket Installation on Cut Slope
- 505 Rock Construction Entrance
- 506 Drop Inlet Protection
- 507 Culvert Control End
- 508 Silt Sock
- 509 Silt Sock
- SERIES 6 WALLS OR MISCELLANEOUS STRUCTURES
- SERIES 7 CURB AND GUTTER AND SIDEWALK
 - 702 Concrete Cross Gutter
 - 703 Curb and Gutter Construction at Catch Basins (B624)
 - 704 Surmountable Concrete Curb & Gutter
 - 705 Curb and Gutter Construction at Catch Basins (B618)
 - 706 Typical Drop Curb Bike Trail
 - 707 Typical Section Bike Trail
 - 708 Concrete Approach Nose Detail

MnDOT 7035L Concrete Walk & Curb Returns at Entrances MnDOT 7100 Concrete Curb & Gutter Design B & V

- SERIES 8 BARRICADES, SIGNALS, MARKERS, ETC.
 - 800 Lateral Offset and Vertical Clearance Type C & D Signs
 - 801 Type C and D Sign Post
 - 802A A-Frame and Stringer Bracing
 - 802B A-Frame and Stringer Bracing
 - 803 Marker Post Detail
 - 805 Stop Sign and Street Name Sign Detail
 - 806 Sign Post Detail
 - 807 Temporary Cul-De-Sac Sign

808 Wetland Buffer Sign Installation

MnDOT 8002 Permanent Barricade

- SERIES 9 MISCELLANEOUS
 - 900 Location of Public Utilities
 - 901 Mail Box Support

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URBAN STREET SECTION 9 TON

LOCAL RESIDENTIAL

SIGMA N18 VALUE IS THE CUMULATIVE DAMAGE EFFECT OF VEHICLES DURING THE DESIGN LIFE OF A FLEXIBLE PAVEMENT.

NOTES: R VALUE IS A MEASURE OF EMBANKMENT SOIL RESISTANCE STRENGTH AS DETERMINED BY THE HVEEM STABILOMETER METHOD.

- NEW CONSTRUCTION ASPHALT BINDER GRADE = B
- MINIMUM ALLOWABLE DESIGN THICKNESS
- * SUBJECT TO REVIEW BY QUALIFIED SOILS ENGINEER

	L	EGEND.			
AASHTO	R VALUE SIGMA N18	BITUMINOUS	SURFACE	AGGREGA	TE BASE
SUBGRADE SOIL CLASS		WEAR 2360***	NON-WEAR 2360***	CLASS 5 OR 6 3138 C*	CLASS 3 OR 4 3138 D*
A-3 A-4 A-6 A-7	$\begin{array}{rrrr} (R-70 &\leq 90,000) \\ (R-20 &\leq 90,000) \\ (R-15 &\leq 90,000) \\ (R-10 &\leq 90,000) \\ (R-5 &\leq 90,000) \end{array}$	** 1 1/2" 1 1/2" 1 1/2" 1 1/2" 1 1/2" 1 1/2"	** 2" 2" 2" 2" 2"	** 8" 8" 8" 8" 8"	_ 12" 18" 24"





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⊐ ≿ _	12.5'		38' FACE	TO FACE		12.5	,
	5' 7'			CL SLOPE 2;	1.5' 19' %	4' SOE SEED BLANI 2% MI	D O ANI KET N.
	SO SO SO SO SO SUBGRADE	D OR SEED D BLANKET K, 5' WIDE E SIDEWALK SIDE) 5 AGGREGATE DVED D	C CLAS	A & B BITUMIN SS 5 AGGREGA SUBGRADE 38'	NOUS MAT TE BASE (MODIFII WIDTH MIN 12"	SLOPI SEE PL/ CONCRE & GUTT ED) 37' WIDTH SUBGRADE PREPA	L TE ER
	AASHTO SUBGRADE	LE R VALUE SIGMA N18	LEGEND N18 BITUMINOUS SURFACE WEAR NON-WEA 2360*** 2360**		AGGREGATE BASE AR CLASS 5 OR 6 CLASS 3 OR 3138 3138		
	JOIL OLASS				C*	D*	
	A-3 A-4 A-6 A-7	$\begin{array}{c} (R-70 \leq 90,000) \\ (R-20 \leq 90,000) \\ (R-15 \leq 90,000) \\ (R-10 \leq 90,000) \\ (R-5 \leq 90,000) \end{array}$	** 1 1/2" 1 1/2" 1 1/2" 1 1/2" 1 1/2" 1 1/2"	** 2" 2" 2" 2" 2"	** 8" 8" 8" 8" 8"	- 12" 18" 24"	
NOTES: R VALUE METHOD SIGMA N	A-3 A-4 A-6 A-7 IS A MEASURE	(R-70 ≤ 90,000) (R-20 ≤ 90,000) (R-15 ≤ 90,000) (R-10 ≤ 90,000) (R-5 ≤ 90,000) * SUBJECT TO REVI ** MINIMUM ALLOWAE *** NEW CONSTRUCTIO E OF EMBANKMENT SOIL HE CUMULATIVE DAMAGE	** 1 1/2" 1 1/2" 1 1/2" 1 1/2" EW BY QUALI BLE DESIGN T ON ASPHALT RESISTANCE S EFFECT OF N	** 2" 2" 2" 2" FIED SOILS EN HICKNESS BINDER GRADE	** 8" 8" 8" 8" NGINEER E = B DETERMINED BY TH	T 12" 18" 24" HE HVEEM STABILO IFE OF A FLEXIBLE	MET
NOTES: R VALUE METHOD SIGMA N PAVEMEN	A-3 A-4 A-6 A-7 IS A MEASURE 18 VALUE IS T T.	$(R-70 \le 90,000)$ $(R-20 \le 90,000)$ $(R-15 \le 90,000)$ $(R-10 \le 90,000)$ $(R-5 \le 90,000)$ * SUBJECT TO REVIN ** MINIMUM ALLOWAE *** NEW CONSTRUCTION E OF EMBANKMENT SOIL HE CUMULATIVE DAMAGE <u>THROUGH</u> <u>URBAN STR</u>	** 1 1/2" 1 1/2" 1 1/2" 1 1/2" EW BY QUALI BLE DESIGN T DN ASPHALT RESISTANCE S EFFECT OF N LOCA CEET SE NO SC	** 2" 2" 2" 2" FIED SOILS EN HICKNESS BINDER GRADE STRENGTH AS /EHICLES DURII L RESIE ECTION CALE	** 8" 8" 8" 8" NGINEER E = B DETERMINED BY TH NG THE DESIGN LI DENTIAL - 9 TON	T 12" 18" 24" HE HVEEM STABILO IFE OF A FLEXIBLE N	MET

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STANDARD PLATE NO. 103

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RESIDENTIAL STREET RURAL

SIGMA N18 VALUE IS THE CUMULATIVE DAMAGE EFFECT OF VEHICLES DURING THE DESIGN LIFE OF A FLEXIBLE PAVEMENT.

St. Francis

NOTES: R VALUE IS A MEASURE OF EMBANKMENT SOIL RESISTANCE STRENGTH AS DETERMINED BY THE HVEEM STABILOMETER METHOD.

- NEW CONSTRUCTION ASPHALT BINDER GRADE = B
- ** MINIMUM ALLOWABLE DESIGN THICKNESS
- * SUBJECT TO REVIEW BY QUALIFIED SOILS ENGINEER

- $(R-20 \leq 90,000)$ $(R-15 \leq 90,000)$ $(R-10 \leq 90,000)$ 1/2" 1/2" 1/2" 1/2" 2" 2" 2" 2" 8" 8" 8" 8" A-4 12" 18" 24" A-61 A-71 (R−5 < 90,000) 1

		D	APPROVED SU	JBGRADE * 32	' WIDTH MIN 12	" SUBGRADE	PREPAF
		CLASS	2 CRUSHED AG	GGREGATE SHO	ULDER (2211) 2	2' WIDE	
AASHTO R VALUE SIGMA N18 BITUMINOUS SURFACE				SURFACE	AGGREGATE BASE		
SUBGRADE SOIL CLASS			WEAR 2360***	NON-WEAR 2360***	CLASS 5 OR 6 3138 C*	CLASS 3 OF 3138 D*	₹ 4
A3	(R-70	< 90.000)	** 1 1/2"	** 2"	** 8"	_	










MINIMUM ROADWAY SECTION	 T.5" SPWEB240B SPNWB230B CLASS 5 AGGREGATE BASE CLASS 5 AGGREGATE BASE CLASS 5 AGGREGATE DASE 	 SPWEB240B SPNWB230B CLASS 5 AGGREGATE BASE CLASS 5 AGGREGATE DASE CLASS 5 AGGREGATE DASE 	 SPWEB240B SPNWB230B SO"/10.0" CLASS 5 AGGREGATE BASE SUBGRADE PREPARATION 	 SPWEB240C SPNWB230B N/10.0" CLASS 5 AGGREGATE BASE SUBGRADE PREPARATION 	4.0" SPWEB240C / SPWEB340C 2.0" SPNWB230F / SPNWB330B 10.0" CLASS 5 AGGREGATE BASE 12.0" SUBGRADE PREPARATION	4.0" SPWEB340F 2.0" SPNWB330B 12.0" CLASS 5 AGGREGATE BASE 12.0" SUBGRADE PREPARATION	TANDARDS
PARKING LANES	0	1	2	2	7	I	IREET S ⁻
ROW WIDTH	50,	60,	66'	80' +	100'+	100, +	UBLIC S
F – F WIDTH (FT)	28,	32,	38,	40'-44' +	VARIES	VARIES	NIMUM
FUNCTIONAL CLASSIFICATION	MINIMUM RESIDENTIAL	RESIDENTIAL STREET	THROUGH RESIDENTIAL STREET	MINOR COLLECTOR	MAJOR COLLECTOR	HIGH DENSITY ARTERIAL	
APPROVED	APPROVEDSTANDARD PLATE NO.REVISEDSTANDARD PLATE NO.108).		

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PRIVATE STREET SECTION ENDING IN CUL-DE-SAC —

LOCAL RESIDENTIAL

SIGMA N18 VALUE IS THE CUMULATIVE DAMAGE EFFECT OF VEHICLES DURING THE DESIGN LIFE OF A FLEXIBLE PAVEMENT.

NOTES: R VALUE IS A MEASURE OF EMBANKMENT SOIL RESISTANCE STRENGTH AS DETERMINED BY THE HVEEM STABILOMETER METHOD.

- NEW CONSTRUCTION ASPHALT BINDER GRADE = B
- MINIMUM ALLOWABLE DESIGN THICKNESS
- * SUBJECT TO REVIEW BY QUALIFIED SOILS ENGINEER

LEGEND					
AASHTO	R VALUE SIGMA N18	BITUMINOUS	SURFACE	AGGREGA	TE BASE
SUBGRADE SOIL CLASS		WEAR 2360***	NON-WEAR 2360***	CLASS 5 OR 6 3138 C*	CLASS 3 OR 4 3138 D*
A-3 A-4 A-6 A-7	$\begin{array}{rrrr} (R-70 &\leq 90,000) \\ (R-20 &\leq 90,000) \\ (R-15 &\leq 90,000) \\ (R-10 &\leq 90,000) \\ (R-5 &\leq 90,000) \end{array}$	** 1 1/2" 1 1/2" 1 1/2" 1 1/2" 1 1/2" 1 1/2"	** 2" 2" 2" 2" 2"	** 8" 8" 8" 8" 8"	- 12" 18" 24"



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STANDARD PLATE NO. 111

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PRIVATE STREET SECTION -THRU

LOCAL RESIDENTIAL

SIGMA N18 VALUE IS THE CUMULATIVE DAMAGE EFFECT OF VEHICLES DURING THE DESIGN LIFE OF A FLEXIBLE PAVEMENT.

NOTES: R VALUE IS A MEASURE OF EMBANKMENT SOIL RESISTANCE STRENGTH AS DETERMINED BY THE HVEEM STABILOMETER METHOD.

- NEW CONSTRUCTION ASPHALT BINDER GRADE = B
- MINIMUM ALLOWABLE DESIGN THICKNESS
- * SUBJECT TO REVIEW BY QUALIFIED SOILS ENGINEER

	LEGEND							
	AASHTO	R VALUE SIGMA N18	IGMA N18 BITUMINOUS SURFACE			AGGREGATE BASE		
:	SUBGRADE SOIL CLASS		WEAR 2360***	NON-WEAR 2360***	CLASS 5 OR 6 3138 C*	CLASS 3 OR 4 3138 D*		
	A-3 A-4 A-6 A-7	$\begin{array}{rrrr} (R-70 &\leq 90,000) \\ (R-20 &\leq 90,000) \\ (R-15 &\leq 90,000) \\ (R-10 &\leq 90,000) \\ (R-5 &\leq 90,000) \end{array}$	** 1 1/2" 1 1/2" 1 1/2" 1 1/2" 1 1/2" 1 1/2"	** 2" 2" 2" 2" 2"	** 8" 8" 8" 8" 8"	- 12" 18" 24"		











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PIPE SIZE	TEE or PLUG	CROSS W/ 2 PLUGS (i.e. 90° BEND)	1/8 BEND (45° BEND) AND 1/16 BEND (22.5°)
6"	0.22 CuYds	0.15 CuYds	0.05 CuYds
8"	0.27 CuYds	0.29 CuYds	0.08 CuYds
10"	0.32 CuYds	0.48 CuYds	0.14 CuYds
12"	0.37 CuYds	0.73 CuYds	0.21 CuYds
16"	0.53 CuYds	1.73 CuYds	0.49 CuYds
20"	0.82 CuYds	3.36 CuYds	0.95 CuYds
24"	1.34 CuYds	5.77 CuYds	1.63 CuYds

NOTE:

- 1. COVER FITTINGS ENCASED IN CONCRETE WITH POLYETHYLENE OR BUILDING PAPER PRIOR TO POURING.
- 2. CONCRETE BLOCKING SHALL BE POURED AGAINST FIRM, UNDISTURBED GROUND.
- CONCRETE SHALL MEET THE REQUIREMENTS FOR GRADE B CONCRETE IN CONFORMANCE WITH Mn/DOT 2461.
- 4. ALL METAL PARTS OF TIE ROD OR STRAP TYPE RESTRAINTS SHALL BE GALVANIZED OR COATED WITH ASPHALTIC TYPE RUSTPROOFING.

WATERMAIN CONCRETE BLOCKING QUANITIES

REVISED

APPROVED



STANDARD PLATE NO. 204













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NOTES:

- 1. THE ADJUSTMENT RINGS AND FRAME SHALL BE SEALED WITH AN 8.5" WIDE DOUBLE PLEATED, OR WIDE, A 10" WIDE TRIPLE PLEATED, INTERNAL CHIMNEY SEAL AS MANUFACTURED BY CRETEX SPECIALTY PRODUCTS. THE SAME EXPANSION BANDS AND EXTENTIONS ARE USED ON BOTH.
- 2. SEE CHIMNEY HEIGHT TABLE FOR SEAL AND EXTENSION COMBINATIONS NEEDED TO SPAN FROM THE FRAME TO THE TOP OF THE CONE ON MANHOLES WITH VARIOUS CHIMNEY HEIGHTS. FRAME OFFSETS OR DIAMETER DIFFERENTIALS WILL REDUCE THESE SPAN HEIGHTS.
- 3. THE TOP OF THE CONE MUST HAVE A MINIMUM 3" HIGH VERTICAL SURFACE THAT IS SMOOTH AND FREE OF ANY FORM OFFSETS OR EXCESSIVE HONEYCOMB. IF A 3" HIGH VERTICAL SURFACE IS NOT AVAILABLE DUE TO THE EXISTING CONFIGURATION OF THE EXISTING MANHOLE CONE, ONE MAY BE CREATED USING A CONE DISK FORM AND A NON SHRINK PATCHING MORTAR. PLANS FOR A FORM DISK CONE ARE AVAILABLE FROM CRETEX SPECIALTY PRODUCTS.

SEAL SELECTION TABLE

COMBINATIONS OF	TO SPAN CHIMNEY HEIGHT OF				
SEALS AND EXTENSIONS	w / standard seal	w / wide seal	W /EXTRA WIDE SEAL		
SEAL ONLY SEAL + 7" EXTENSION SEAL + 10" EXTENSION	0"-4.5" OVER 4.5"-10.5" OVER 10.5"-13"	2"-7.5" OVER 7.5"-13.5" OVER 13.5"-16"	OVER 6"-12" OVER 12"-18" OVER 18"-20.5'		
SEAL + MULT. EXTENSION	OVER 13''	OVER 16"	OVER 20.5"		
ADD 6" OF COVERAGE FOR FACH ADDITIONAL 7" EXTENSION					

ADD 6 OF COVERAGE FOR EACH ADDITIONAL 7 EXTENSION ADD 8.5' OF COVERAGE FOR EACH ADDITIONAL 10' EXTENSION DIAMETER DIFFERENTIALS AND OFFSETS WILL REDUCE THESE COVERAGES

INTERNAL CHIMNEY SEAL

APPROVED

REVISED

St. Francis

STANDARD PLATE NO. 308









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	POUR CONCRETE COLLAR AROUND FES TO ANCHOR BLOCKS							
	NOTE: DIMENSIO STD. PL.	N E IS GIVEN 3100 & 311	1 ON 2'1 0.	BLOCK BE BACK W/TC	SHALL FILLED DPSOIL	$B \left(\begin{array}{c} \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\$		
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			GEOTEXTILE	USE EQ STD.	4 5D UIV. D FOR PIPE PL. 3110 FOR 1.0' ① -	B ARCH (SEE EQUIV. D) CONCRETE BLOCK		
	SEC A-A ARTICULATED CONCRETE BLOCK SHALL BE A HANDPLACED INTERLOCKING CONCRETE BLOCK SYSTEM, CABLE CONNECED CONCRETE BLOCK MAT, OR APPROVED EQUAL. (1) FOR PIPES GREATER THAN OR EQUAL TO 48", USE 2.0' (2) GEOTEXTILE FABRIC PER Mn/DOT SPEC. 3733. FABRIC SHALL BE OVERSIZED A MINIMM OF 12" AND COVER THE ENTIRE AREA UNDER THE BLOCK MAT AND EXTEND UNDER THE CULVERT APRON 3 FEET. (3) IF A CABLE CONCRETE SYSTEM IS USED, MULTIPLE MATS MUST BE TIED TOGETHER PER MANUFACTURERS SPEC. AND ALL CABLES PROTRUDING BEYOND THE FINISHED EDGES SHALL BE CUT FLUSH TO THE BLOCK. <u>ARTICULATED CONCRETE BLOCK AT R.C.P. OUTLET</u> NO SCALE							
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r									
LEGEND									
AASHTO	R VALUE	BITUMINOUS SURFACE	AGGREGATE BASE						
SUBGRADE SOIL CLASS		WEAR 2360*** A	CLASS 5 3138 B*	CLASS 3 OR 4 3138 C*					
A-3 A-4 A-6 A-7	R-70 R-20 R-15 R-10 R-5	** 3" 3" 3" 3" 3"	** 6" 6" 8" 6"	- - - 12"					

* SUBJECT TO REVIEW BY QUALIFIED SOILS ENGINEER ** MINIMUM ALLOWABLE DESIGN THICKNESS *** ASPHALT BINDER GRADE = B

NOTES: R VALVE IS A MEASURE OF EMBANKMENT SOIL RESISTANCE STRENGTH AS DETERMINED BY THE HVEEM STABILOMETER METHOD.

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APPENDIX B

POLICY ON STORMWATER DRAINAGE SUBMITTAL REQUIREMENTS FOR DEVELOPERS

CITY OF ST. FRANCIS

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- 1.0 Purpose and Intent
- 2.0 Incorporation by Reference
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 - B. Rainfall
 - C. Curve Numbers
 - D. Flood Protection
 - E. On-Site Detention Basins
 - F. Storm Sewer
- 5.0 General Requirements Grading, Drainage, and Erosion Control Plan
- 6.0 Storm Drainage System Submittal Requirements
- 7.0 Glossary

TABLE 1CITY OF ST. FRANCIS MINIMUM RUNOFF CURVE NUMBERS

POLICY ON STORMWATER DRAINAGE SUBMITTAL REQUIREMENTS FOR DEVELOPERS

CITY OF ST. FRANCIS

1.0 Purpose and Intent

This policy is intended to provide Developer's Engineers with a standardized format for submittal of drainage plans and calculations to the City for review. A standardized format will provide the following:

- Reduce preparation time for submittals by providing direct guidelines for Developer's Engineers to follow
- Reduce review time required by the City's Engineer by insuring a complete and comprehensive drainage plan and calculations are submitted
- Insure that the City will receive the best possible protection of its resources, which could be adversely affected by inadequate stormwater management planning.

2.0 Incorporation by Reference

Protecting Water Quality in Urban Areas (Best Management Practices for Minnesota) prepared by the Minnesota Pollution Control Agency, Division of Water Quality, latest edition, shall be incorporated by reference into this policy.

The Minnesota Stormwater Manual as published online and available here: <u>Minnesota Stormwater Manual (state.mn.us)</u> shall be incorporated by reference.

Recommendations set forth in the above referenced manual shall be implemented by the Developer's Engineer. All recommendations set forth within the above referenced manual shall be termed "required" when applicable unless otherwise amended by this policy.

3.0 State and Federal Requirements

State and Federal Ordinances, Codes, Regulations, and Requirements shall be adhered to by the Developer.

4.0 Calculations and Considerations

A. General Hydrology

Hydrologic analysis of storm water runoff for the planning and design of flows in storm sewers, ditches, streams and channels to lakes, detention basins, and wetlands shall be made using generally accepted hydrograph methods.

Determination of total runoff volume should follow the USDA-SCS curve number method which incorporates land use and hydrologic soil groups. Specific step-by-step process can be found in the Soil Conservation Service (SCS) publication National Engineering Handbook: Chapter 4, SCS Hydrology (1972), and Hydrology Guide for Minnesota (1992). Peak runoff rates should be determined through the use of the SCS method incorporating "time of concentration" for both pre and post development conditions.

Then the storm water should be routed through the drainage area, that is, mathematically the peaks and volumes are followed as they move in a wave progressively downstream.

"Design Storms" or storm volumes for hydrologic analyses shall be based upon Hershfield, D.M., 1961, Rainfall Frequency Atlas of the United States for Durations of 30 minutes to 24 hours and Return Periods from 1 to 100 years, Technical Publication Number 40 (TP-40) along with the supplementary documents entitled: Oberts, G. L., 1984, Surface Water Management: Precipitation Frequency Analysis for the Twin Cities Metropolitan Area, Metropolitan Council, Publication Number 10-84-007 and Fredrick, R.H., 1977, Five-to-Six-Minute Precipitation Frequency for the Eastern united States, NOAA Technical Memorandum NWS HYDRO-35, Office of Hydrology, Silver Spring, Maryland.

The rational method may be used to determine peak runoff rates for primary systems. Construction of a hydrograph should be undertaken which characterizes the movement of surface water as a function of time and precipitation. Rainfall intensity shall be determined by using the IDF curves in the Mn/DOT Drainage Manual dated August 30, 2000.

Minimum time of concentration shall be 10 minutes for drainage areas with tributary areas, 7 minutes without tributary areas. When a portion of the drainage area is highly impervious, the drainage area shall be evaluated both with and without tributary area to verify that just the highly impervious area does not result in greater peak discharge than the area evaluated as a whole.

B. Rainfall

Usually the standard 24-hour SCS rainfall distribution will be used to calculate the peak discharge rates and levels. The following rainfall values shall be used in calculations for the City of St. Francis:

Event

Rainfall (inches)

1 year, 24 hour	2.44
2 year, 24 hour	2.85
10 year, 24 hour	4.22
25 year, 24 hour	5.21
50 year, 24 hour	6.04
100 year, 24 hour	6.94

C. Curve Numbers

Table 1 lists the minimum allowable Curve Numbers (CN) which shall be used for design. Hydrologic soil groups shall be determined based upon the Soil Survey for Anoka County, Minnesota as published by the United States Department of Agriculture Soil Conservation Service in Cooperation with Minnesota Agricultural Experiment Station.

D. Flood Protection

Consistent with state and federal regulations, the City of St. Francis requires that the level of flood protection along all ditches, detention basins, lakes, streams and wetlands be established based upon the 1 percent (100-year frequency) flood. Land use within floodplains shall be regulated in accordance with state floodplain zoning regulations.

The following freeboard values are required for the City of St. Francis:

•	Landlocked Basins (no outlet)	3 feet	(Established	high
	water,		see 4.E.8.)	
•	Non-landlocked basins	1.5 feet	(100-year	
			frequency)	

E. On-Site Detention Basins

It is the policy of the City of St. Francis to require developments to control storm water quantity and quality through a management approach of detention basins. Detention basins, whether on-site or regional in nature, shall be designed to incorporate the following:

1. A permanent pool ("dead storage") volume below the normal elevation which shall be greater than or equal to the runoff from a 2.5-inch rainstorm over the entire contributing drainage area assuming full development. This modified NURP criteria includes a 25 percent increase in basin storage to permit routine sediment accumulation over a 20-year design period, assuming the drainage area is protected with proper erosion and sedimentation control practices. The runoff volume shall consider the entire area contributing to the pond, however, the minimum permanent pool volume must be greater than or equal to the volume produced from

0.5 inches of runoff from all impervious area in the contributing watershed.

- 2. A permanent pool average depth (basin volume/basin area) which shall be greater than 4 feet with a maximum depth of less than 10 feet.
- 3. An emergency spillway (emergency outlet) adequate to control the one percent frequency/duration rainfall event (usually 100-year, 24-hour).
- 4. Basin side slopes above the normal water level should be no steeper than 4:1, and preferably flatter. A basin shelf with a minimum width of 10 feet and a slope of 10:1 starting at the normal water level.
- 5. To prevent short-circuiting, the distance between major inlets and the normal outlet shall be maximized. The ratio of maximum length to maximum width of the permanent pool should be at least 3:1.
- 6. To protect downstream channels and structures, the following flood control criteria are required for basin design:
 - a. A flood pool ("live storage") volume above the normal elevation shall be adequate so that the peak discharge rates from the 2-year and 100-year frequency, critical duration storms (usually the 24-hour) are no greater than predevelopment basin watershed conditions.
 - b. Storage volumes and discharge rates have been established for the 100-year event for certain portions of the city. In these areas the established storage volumes and discharge rates shall be used for post development design.
 - c. Dead storage volume may not be utilized as live storage.
- 7. Skimming structures shall be utilized for each basin. The skimming structure shall be in accordance with the City Standard Plates. Skimming structures shall be shown on the plans.
- 8. Where discharge from the basin is not possible, the permanent basin must be sized for two 100 year events back-to-back. In this situation the free board above the established high water level shall be a minimum of three (3) feet. The high water level shall be established as follows:
 - a. Assume the water surface is at the normal water surface elevation of the basin.

- b. Above the assumed water surface elevation store the volume of runoff equal to two 100-year, 24-hour storm events over the entire drainage area to the landlocked basin.
- c. The established high water level is the elevation the water would rise to from the above steps a and b.
- 9. Discharge must be made to a receiving stream, ditch, or another pond or an approved discharge route as shown in the Storm Water Management Plan.
- F. Infiltration / Filtration Basins
 - 1. All infiltration/filtration basins shall be designed and constructed in accordance with the Minnesota Pollution Control Agency's standards and City of St. Francis ordinance requirements.
 - 2. Pre-treatment shall be provided prior to stormwater entering the infiltration/filtration basins.
 - a. For publicly maintained infiltration/filtration basins, the pretreatment shall consist of a sediment forebay designed in accordance with the MN Stormwater manual recommendations.
 - 3. On-site soil testing (i.e. soil borings and/or double ring infiltrometer tests) shall be preformed within each infiltration/filtration basin.
- G. Storm Sewer
 - 1. Storm sewer sizing shall be based upon the 10 year storm event. Inlet capacities and roadway spread at each inlet shall be determined. The maximum allowable roadway spread at any inlet shall be one-half of the traveled lane.

Storm sewer inlets shall be spaced to insure that not more than $\frac{1}{2}$ of the traveled lane is inundated during the 10 year storm event. Manning's equation shall be utilized to determine the flow in the street at each catchbasin for verification of actual spread. A manning's n of 0.016 shall be utilized for asphalt pavement. Additionally, grate inlet capacities shall be verified at the maximum allowable depth of flow to verify that the proposed grates will pass the 10 year flows. When appropriate, by-pass flows shall be considered in calculations.

- 2. Storm sewer systems shall also meet the following requirements:
 - a. Maintain a minimum velocity of 3 fps for 10-year storm event.

- b. Maintain a minimum cover of 2 feet from top of pipe to top of casting or flow line elevation.
- c. Maintain a minimum of 3 feet of final cover over corrugated high density polyethylene (HDPE) pipe. See engineering guidelines to determine when HDPE is allowed.
- d. Maintain a minimum of 1.5 feet of final cover over RCP in areas not used for vehicle traffic.
- e. Storm sewers inverts, which outlet to detention basins, shall be placed at the normal water elevation of the basin. Storm sewers may be submerged a maximum of 1/2 the pipe diameter below the basin normal water elevation.

5.0 General Requirements - Grading, Drainage, and Erosion Control Plan

Grading, Drainage, and Erosion Control Plans shall be provided by the Developer in accordance with this manual. Several items critical to the review of the drainage system must be adequately depicted on the plan by the Developer's Engineer. The following key elements must be depicted on the plan:

- A. Existing and proposed contours at a minimum of 2-foot intervals. A 1-foot contour interval or proposed spot elevations shall be used where conditions dictate. The determination of contour interval shall be made based upon clarity and readability of the plans.
- B. Basin locations as depicted by the proposed contours. Normal level and 2 year, 10 year and 100 year flood water levels shall be depicted on the plan for each basin. Detention basins are required at each ditch and storm sewer outfall point from the proposed plat. Perimeter berm elevation and width shall be clearly labeled on plan sheets.

Permanent detention basins may be utilized as construction detention basins, provided they are cleaned after permanent erosion control measures are established. Design features of the detention ponds shall be as described in the BMP Manual.

- C. Locations of silt fence, bale checks, erosion control blanket, rock construction entrances, storm drain inlet protection, outlet projection, rip rap, temporary seeding, permanent seeding, sod, mulch, or other erosion control features proposed to be implemented for the project.
- D. Storm sewer facilities, when utilized, shall be adequately depicted on the drawings. As a minimum, the following must be shown on the plan:
 - 1. Storm sewer pipe length, grade, type of material, and size between each catch basin and manhole.

- 2. Catchbasin and manhole structural data including size or diameter, and depth. A typical section depicting each different type of catchbasin or manhole used shall be shown on the drawing. Type of casting utilized shall be referenced for each catchbasin or manhole. Elevations for the top of inlet and each invert shall be referenced on the drawing.
- 3. A typical curb section for urban design streets shall be shown on the drawing.
- 4. If ditch sections are used, a typical section shall be shown on the drawing depicting bottom width and side slopes of the ditch.
- 5. Details of skimming structures utilized.
- E. Individual lot grading shall insure positive drainage. Lot grading shall clearly depict a minimum design slope of 2%. Slopes of 1% to 2% may be allowed on a case by case basis with approval from the City Engineer. Under no circumstances will slopes less than 1% be allowed.

6.0 Storm Drainage System Submittal Requirements

- A. The stormwater drainage report shall be comprised of the following sections to provide the City Engineer with adequate base information for which to review the report. The following data must be included in the report:
 - 1. Title Page. The title page shall list the project name, project location, date prepared, and preparer's name, title, and company.
 - 2. Signature Page. The report shall be signed by a licensed professional engineer.
 - 3. Table of Contents. The table of contents must provide a description of the major categories of the report and also list each hydrograph and reservoir report presented in the report.
 - 4. Stormwater Summary. The summary must provide descriptions of items critical to the review of the entire report. Assumptions and results of the calculations shall be included in the summary. As a minimum, the following items must be discussed in the summary:
 - a. Pre-development site conditions (Existing)
 - i. Total site area
 - ii. Delineation of sub-drainage areas, as appropriate.
 - iii. For each drainage area, or sub–drainage area, provide the following information:
 - 1. Area in acres

- 2. Curve number (with justification)
- 3. Time of Concentration (with justification)
- 4. Runoff rate and runoff volume
- b. Post Development Site Conditions (Proposed)
 - i. Total site area
 - ii. Delineation of sub-drainage areas, as appropriate.
 - iii. For each drainage area, or sub–drainage area, provide the following information:
 - 1. Area in acres
 - 2. Curve number (with justification)
 - 3. Time of Concentration (with justification)
 - 4. Runoff rate and runoff volume
- b. Comparison of pre-development to post-development runoff rates and volumes.
- c. Comparison of infiltration volume required to infiltration volume provided.
- d. Discussion of temporary and permanent erosion control measures utilized.
- e. A discussion of the storm sewer system, if applicable, to include a summary of flows to each catchbasin and the depth of water over each catchbasin during the ten year event.
- 5. Drainage maps depicting pre-development and post-development conditions. The maps may be 22"x34" plans, but shall also be provided on 11"x17" reductions. The plans shall delineate drainage area and sub-drainage area boundaries. All areas shall be labeled and referenced to those presented in the report.
- 6. Computer printouts of all hydrograph and reservoir files shall be included at the back of the report for reference.

7.0 Glossary

Critical Storm

Critical Storm means that rainfall event whose distribution and duration results in a runoff volume and rate establishing the appropriate level of protection.

Freeboard

Is the vertical difference between the lowest floor of proposed buildings and the critical 100-year storm event elevation or established high water level.

Level of Protection

The amount of secondary storm water runoff capacity required to avoid flood damage and provide for public safety.

Level of Service

The amount of primary storm water runoff capacity required to avoid unusual hardship or significant interference with normal public activities (transportation, sanitary, or utilities).

Normal Level

For basins, that water elevation maintained by a natural or man-made outlet.

<u>NURP</u>

Nationwide Urban Runoff Program (USEPA, 1983).

100-Year Storms

Rainstorms of varying duration (e.g. 2-, 6-, 24- or 48-hour) and intensities expected to recur on the average of once every one hundred years (1% frequency probability).

On-Site Detention

A method of temporarily storing storm water runoff at a development site in the form of wet basins.

Primary Capacity

The volume and/or rate of storm water runoff defined as that level of service provided by the primary system.

Primary System

The primary system conveys runoff from the more frequent events such as the 2 to 10-year events. In general, the system is composed of swales, ditches, gutters, and storm sewers.

Secondary Capacity

The volume and/or rate of storm water runoff in excess of the primary capacity and defined as that level of protection provided by the secondary system.

Secondary System

The system is composed of all the pathways that runoff takes when the capacity of the primary system is exceeded and in general is composed of streets, swales, ditches, stormsewers, detention basins, creeks, streams and rivers.

Storm Water Runoff

The flow on the surface of the ground, resulting from precipitation in the form of rainfall or snowmelt.

Table 1					
City of	St. Francis	Minimum	Runoff (Curve N	umbers

Cover Description	Curve numbers for hydrologic soil group			
Cover type and hydrologic condition	Α	В	C	D
Fully developed urban areas (vegetation	established)			
Open space (lawns, parks, golf courses,				
cemeteries, etc.				
Grass Cover > 75%	39	61	74	80
Grass Cover < 75%	49	65	77	82
Impervious areas:				
Paved parking lots, roofs,				
driveways, etc. (excluding right-of-				
way)	98	98	98	98
Streets and roads:				
Paved; curbs and storm sewers				
(excluding right-of-way)	98	98	98	98
Paved; open ditches (including				
right-of-way)	83	89	92	93
Gravel (including right-of-way)	76	85	89	91
Dirt (including right-of-way)	72	82	87	89
Water Surface:	100	100	100	100
Urban Districts:				
Commercial and business	NA ¹	92	94	95
Industrial	NA ¹	88	91	93
Residential districts by average lot size:				
1/8 acre of less (town houses)	NA^1	85	90	92
1/4 acre	NA ¹	75	83	87
1/3 acre	NA ¹	72	81	86
1/2 acre	NA ¹	70	80	85
1 acre	59	68	79	84
2 acres and greater	55	65	77	82
Developing Urban Areas				
Newly graded areas (pervious areas				
only, no vegetation)	77	86	91	94
Undeveloped areas				
Agricultural land (all current uses)	55	65	77	82
Pasture, grassland, or range –				
continuous forage for grazing	49	65	77	82
Meadow – continuous grass,				
protected from grazing and				
generally mowed for hay	30	58	71	78
Brush – brush-weed-grass mixture				
with brush the major element	35	56	70	77
Woods – grass combination		· -		0.2
(orchard or tree farm)	43	65	76	82
337 1	27	<i>(</i> 0	70	70
Woods	36	60	73	79

¹Use of Type A soil is not allowed for this hydrologic condition